Point-wise impulse (blast) response of a composite sandwich plate including core compressibility effects

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ABSTRACT

This work analyzes the nonlinear impulse response of a composite sandwich plate exposed to a sudden point-wise transverse loading on the top face sheet. The nonlinearity arising from the core compressibility in the thickness direction is modeled and incorporated into the constitutive relations explicitly. As such, one can have a deep insight regarding the stress, strain and displacement profiles into the sandwich plate. The sandwich plate is assumed to be perfectly bonded at the face sheet/core interfaces. The equations of motion are formulated using Hamilton’s principle. The simply supported case is used to illustrate the procedure for solving the nonlinear equations. Numerical results are presented to demonstrate the response in terms of the transverse deformation and stresses in the composite sandwich plate. The effects of the variation of the geometrical parameters of the structure on the blast impulse response are also studied. Some conclusions are suggested regarding the associated optimal design of sandwich plates.

1. Introduction

A typical sandwich plate consists of two stiff metallic or composite thin face sheets separated by a soft honeycomb or foam thick core of low density. This configuration gives the sandwich material system high stiffness and strength with little resultant weight penalty and high-energy absorption capability related to the application of sandwich structures in the construction of aerospace vehicles, naval vehicles and civil infrastructure. Most of the studies in sandwich composites neglect the transverse deformation of the core as mentioned in the Sandwich Structures books (Plantema, 1966; Allen, 1969; Vinson, 1999). The core of a sandwich structure is treated as infinitely rigid in the thickness direction and only its shear stresses are taken into account. This assumption works well in the analysis of sandwich structural response to a static or dynamic loading of long-duration. However, several studies (Kwon and Lannamann, 2002; Xue and Hutchinson, 2004; Fleck and Deshpande, 2004; Li et al., 2008) have shown that the core transverse deformation/strain in a sandwich structure subject to impulsive loading has a highly non-linear profile with respect to the thickness-wise coordinate. Therefore, it has been shown that the non-linear high order core theory in Li and Kardomeas (2008) is very accurate, yielding essentially identical results to the elasticity solution for static transverse loading. Therefore, this paper shall extend this non-linear core model in to address the dynamic response of sandwich plates subject to point-wise blast impact loading. Consideration of the core compressibility implies that the displacements of the top and bottom face sheets may not be identical. The following assumptions will be adopted in this paper: (1) the face sheets satisfy the Kirchhoff-Love assumptions and their thicknesses are small compared with the overall thickness of the sandwich section, and the two face sheets are further assumed to have identical thickness; (2) the core is compressible in the transverse direction, that is, its thickness may change; (3) the bonding between the face sheets and the core is assumed perfect; and (4) an impulse loading decaying exponentially with time (blast loading) applied at a specific point on the top face of the plate will be considered.

This paper is organized as follows: the high order non-linear transverse compressible core theory assumptions are summarized in Section 2. The equations of motion and boundary and initial conditions are formulated in Section 3 via Hamilton’s principle. These unknowns in the equations are highly coupled in terms of both the spatial and time variables. A solution procedure for solving the non-linear partial governing equations is formulated in Section 4 using the simply supported case as an example. Results from point-wise sudden impulse loading on the top face sheet of a sandwich plate are presented and discussed in Section 5. Suggested in Section 6 are some conclusions.

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2. Formulation

2.1. Kinematic relations

In the following, we consider a sandwich plate with two identical face sheets of thickness \( h_t \) and a core of thickness \( h_c \) and let a cartesian coordinate system \( (x, y, z) \) be on the middle plane of the core, as shown in Fig. 1. The corresponding displacements are denoted by \((u, v, w)\). We further use the superscript “\(t, b, c\)” to refer to the top face sheet, bottom face sheet or core, respectively, and the subscript "0" to refer to the middle surface of the corresponding phase.

The face sheets are assumed to satisfy the Kirchhoff-Love assumptions and their thicknesses are small compared with the overall thickness of the sandwich section. Therefore, if we define

\[ \zeta = z \pm \left( \frac{h_t}{2} + \frac{h_c}{2} \right), \]

we can write the displacement relations for the face-sheets as:

\[ u^b(x, y, z) = u^b_t(x, y) - \zeta w^b_t(x, y), \]
\[ v^b(x, y, z) = v^b_t(x, y) - \zeta w^b_t(x, y), \]
\[ w^b(x, y, z) = w^b_t(x, y), \quad -\frac{h_c}{2} \leq \zeta \leq \frac{h_c}{2}. \]

Omitting the superscripts \( t \) and \( b \), the non-linear strain-displacement relations for the face-sheets can take the following form:

\[ [e] = \begin{bmatrix} \epsilon_{xx} \\ \epsilon_{yy} \\ \gamma_{xy} \end{bmatrix} = \begin{bmatrix} \epsilon^0 + \zeta k_x \\ \epsilon^0 + \zeta k_y \\ \gamma^0_{xy} + \zeta k_y \end{bmatrix}, \]

in which \([\epsilon^0]\) is the middle surface strain given by

\[ [\epsilon^0] = \begin{bmatrix} \epsilon^0_{xx} \\ \epsilon^0_{yy} \\ \gamma^0_{xy} \end{bmatrix} = \begin{bmatrix} u_{0,xx} + \frac{1}{2} w^2_{xx} \\ u_{0,yy} + \frac{1}{2} w^2_{yy} \\ u_{0,xy} + w_{xx} w_{yy} \end{bmatrix}. \]

Moreover, \([k]\) is the curvature

\[ [k] = \begin{bmatrix} k_x \\ k_y \\ -w_{xx} \\ -w_{yy} \end{bmatrix}. \]

During the impulsive loading process, the core may undergo a considerable reduction in thickness. In order to capture this core transverse compressibility, we use a higher order core theory as formulated by Li and Kardomeatas (2008). In this theory, the transverse displacement in the core, \( w^c \), is of fourth order in the transverse direction \( z \):

\[ w^c(x, y, z) = \left( 1 - \frac{2z^2}{h_c} \right) w_0^c(x, y) + \left( \frac{2z^2}{h_c} \right) w_0^c(x, y) \]
\[ - \left( \frac{z}{h_c} \right) \frac{4z^3}{h_c}, \]

in which \( w_0^c(x, y) \) is the transverse displacement of the middle surface of the core, and \( w(x, y) \) and \( w(x, y, z) \) are, respectively, the average and difference of the middle surface transverse displacements for the two face-sheets,

\[ w(x, y) = \frac{1}{2} [w^t(x, y) + w^b(x, y)], \]
\[ w(x, y) = \frac{1}{2} [w^t(x, y) - w^b(x, y)]. \]

The in-plane displacements in the core, \( u^c \) and \( v^c \), are of fifth order in \( z \), expressed as follows:

\[ u^c(x, y, z) = \frac{1}{2} \left( u^t_0(x, y) + u^b_0(x, y) \right), \]
\[ u^c(x, y, z) = \frac{1}{2} \left( u^t_0(x, y) - u^b_0(x, y) \right), \]
\[ v^c(x, y, z) = \frac{1}{2} \left( v^t_0(x, y) + v^b_0(x, y) \right), \]
\[ v^c(x, y, z) = \frac{1}{2} \left( v^t_0(x, y) - v^b_0(x, y) \right). \]

These displacement profiles satisfy the displacement continuity, at the top face sheet/core interface, \( z = -h_c/2 \) and at the bottom face sheet/core interface, \( z = h_c/2 \).

It should be noted that like any other plate theory, this is still an approximate model and, although the displacement field satisfies all continuity and compatibility conditions, the equilibrium equations may not be satisfied within the core. However, a validation study for the static loading case has shown that this high order theory gives a displacement distribution almost exactly as the elasticity solution and a transverse stress distribution most close to the elasticity solution among all current sandwich theories (Li and Kardomeatas, 2008).

The displacement profiles postulated above lead to the following strain relations for the core:

\[ \epsilon_{xx} = \left( -\frac{1}{2h_c} \frac{2z}{h_c} \frac{6z}{h_c} + \frac{16z^3}{h_c} \right) w_0(x, y) + \left( \frac{1}{2h_c} \frac{2z}{h_c} \frac{6z}{h_c} + \frac{16z^3}{h_c} \right) w^c(x, y), \]
\[ \gamma_{xy} = -\frac{2}{h_c} \left( u^c(x, y) + \eta_1(z) w_0^c(x, y) + \eta_2(z) w_0^c(x, y) + \eta_3(z) w_0^c(x, y) \right), \]
\[ \gamma_{xy} = -\frac{2}{h_c} \left( u^c(x, y) + \eta_1(z) w_0^c(x, y) + \eta_2(z) w_0^c(x, y) + \eta_3(z) w_0^c(x, y) \right), \]

in which,

![Fig. 1. A composite sandwich plate subject to point-wise blast (impulse) loading.](image-url)
\( \eta_1(z) = -\left(1 + \frac{2h_l}{h_c} \right) \frac{z}{2h_c} + \left(1 + \frac{3h_l}{h_c} \right) \frac{z^2}{h_c^2} - \left(1 + \frac{4h_l}{h_c} \right) \frac{2z^3}{h_c^3} \)
\[ + \left(1 + \frac{5h_l}{h_c} \right) \frac{4z^4}{h_c^4}. \]
\( \eta_2(z) = \left(1 + \frac{h_l}{h_c} \right) + \left(1 + \frac{3h_l}{h_c} \right) \frac{2z^2}{h_c^2} - \left(1 + \frac{5h_l}{h_c} \right) \frac{8z^4}{h_c^4} \]
\( \eta_3(z) = \left(1 + \frac{2h_l}{h_c} \right) \frac{z}{2h_c} + \left(1 + \frac{3h_l}{h_c} \right) \frac{z^2}{h_c^2} + \left(1 + \frac{4h_l}{h_c} \right) \frac{2z^3}{h_c^3} \)
\[ + \left(1 + \frac{5h_l}{h_c} \right) \frac{4z^4}{h_c^4}. \]
\( \delta T = \int_0^t \int_{-b/2}^{b/2} \int_{-a/2}^{a/2} \rho \left(\dot{u}_x \dot{\delta} u_x + \dot{u}_y \dot{\delta} u_y + W \dot{\delta} \omega \dot{\omega}\right) \, dz \, dx \, dy \, dt, \]
\( \delta U = \int_0^t \int_{-b/2}^{b/2} \int_{-a/2}^{a/2} \left(\sigma_{xx} \dot{e}_{xx} + \sigma_{yy} \dot{e}_{yy} + \sigma_{xy} \dot{\gamma}_{xy}\right) \, dz \, dx \, dy \, dt, \]
\( \delta W = \int_0^t \int_{-b/2}^{b/2} \int_{-a/2}^{a/2} q(x,y,t) \omega \dot{\omega} \, dx \, dy \, dt, \)

It should be noted that the core is considered undergoing large rotation with a small displacement, therefore, the in-plane strains can be neglected.

### 2.2. Constitutive relations

The equations developed so far can be applied to general materials. In the following sections, we will assume the face sheets are made of orthotropic laminated composites and the core is also orthotropic.

The general stress–strain relationship for any layer of the face sheets reads as:
\[ \begin{bmatrix} \sigma_{xx} \\ \sigma_{yy} \\ \tau_{xy} \end{bmatrix} = \begin{bmatrix} C_{11} & C_{12} & C_{16} \\ C_{12} & C_{22} & C_{26} \\ C_{16} & C_{26} & C_{66} \end{bmatrix} \begin{bmatrix} e_{xx} \\ e_{yy} \\ \gamma_{xy} \end{bmatrix}, \]

where \( C_{ij} \) for \( i,j = 1,2,6 \) are the plane-stress reduced stiffness coefficients. Therefore, based on classic laminated composite theory, one can find the resultants for the top or bottom face sheet of a sandwich plate as:
\[ \begin{bmatrix} N_x \\ N_y \\ N_{xy} \end{bmatrix} = \begin{bmatrix} A_{11} & A_{12} & A_{16} \\ A_{12} & A_{22} & A_{26} \\ A_{16} & A_{26} & A_{66} \end{bmatrix} \begin{bmatrix} e_x \\ e_y \\ \gamma_{xy} \end{bmatrix} + \begin{bmatrix} B_{11} & B_{12} & B_{16} \\ B_{12} & B_{22} & B_{26} \\ B_{16} & B_{26} & B_{66} \end{bmatrix} \begin{bmatrix} k_x^1 \\ k_y^1 \\ k_{xy}^1 \end{bmatrix}, \]

for the resultant force and
\[ \begin{bmatrix} M_x \\ M_y \\ M_{xy} \end{bmatrix} = \begin{bmatrix} B_{11} & B_{12} & B_{16} \\ B_{12} & B_{22} & B_{26} \\ B_{16} & B_{26} & B_{66} \end{bmatrix} \begin{bmatrix} e_x \\ e_y \\ \gamma_{xy} \end{bmatrix} + \begin{bmatrix} D_{11} & D_{12} & D_{16} \\ D_{12} & D_{22} & D_{26} \\ D_{16} & D_{26} & D_{66} \end{bmatrix} \begin{bmatrix} k_x^0 \\ k_y^0 \\ k_{xy}^0 \end{bmatrix}, \]

for the resultant moment. The \( A \) (extensional), \( B \) (coupling) and \( D \) (bending) stiffness matrices are respectively defined as:
\[ [A_{ij}, B_{ij}, D_{ij}] = \begin{cases} \frac{1}{2} C_{ij} \times \{1, z^2\} \, dz & \text{for top face} \\ \int_{-c}^{c} C_{ij} \times \{1, z^2\} \, dz & \text{for bottom face} \end{cases} \]

The stress–strain relations for an orthotropic core can be written as:
\[ \sigma_{zz} = E' \varepsilon_{zz}, \quad \tau_{xz} = G' \varepsilon_{xz}, \quad \tau_{yz} = G' \varepsilon_{yz}. \]

### 3. Equations of motion

The equations of motion and appropriate boundary conditions can be derived using the Hamilton’s principle. The sandwich plate is subject to an impulsive transverse loading \( q(x,y,t) \) on the top face-sheet. Let the strain energy be denoted by \( U \), the external potential by \( W \) and the kinetic energy by \( T \), then the variational principle states:
\[ \delta[T - U + W] = 0, \]
in which,
\[ \delta T = \int_0^t \int_{-b/2}^{b/2} \int_{-a/2}^{a/2} \rho \left(\dot{u}_x \dot{\delta} u_x + \dot{u}_y \dot{\delta} u_y + W \dot{\delta} \omega \dot{\omega}\right) \, dz \, dx \, dy \, dt, \]
\[ \delta U = \int_0^t \int_{-b/2}^{b/2} \int_{-a/2}^{a/2} \left(\sigma_{xx} \dot{e}_{xx} + \sigma_{yy} \dot{e}_{yy} + \sigma_{xy} \dot{\gamma}_{xy}\right) \, dz \, dx \, dy \, dt, \]
\[ \delta W = \int_0^t \int_{-b/2}^{b/2} \int_{-a/2}^{a/2} q(x,y,t) \omega \dot{\omega} \, dx \, dy \, dt, \]
\[
- \alpha_1 h_t (G_{zz} w^{v}_{xx} + G_{zz} w^{v}_{yy}) - \alpha_0 (G_{zz} (u^{v}_{xx} - u^{v}_{yy})) \\
+ G_{zz} (v^{v}_{xy} - v^{v}_{yy}) - \frac{61}{21} \frac{E_r}{h_t} \left( w^{p}_{x} - \frac{358}{305} w^{p}_{y} + \frac{53}{305} w^{w} \right) \\
+ q(x, y, t) = 0,
\]

in which \( \alpha_i \) (i = 1, ..., 4) are constants in terms of the ratio of face thickness and core thickness as follows:

\[
\begin{align}
\alpha_0 &= \frac{2}{15} \frac{h_t}{h_c}, \quad \alpha_1 = \frac{29}{315} \frac{h_t}{h_c} + \frac{247}{252} \left( \frac{h_t}{h_c} \right)^2, \\
\alpha_2 &= \frac{37}{630} + \frac{37}{630} \frac{h_t}{h_c}, \quad \alpha_3 = \frac{11}{630} + \frac{11}{180} \left( \frac{h_t}{h_c} \right)^2.
\end{align}
\]

The equation of motion for the compressible core is:

\[
\begin{align}
\partial_w \psi_0 &\quad \alpha_4 h_t (G_{zz} w^{v}_{xx} + G_{zz} w^{v}_{yy}) + \alpha_0 h_t (G_{zz} (w^{v}_{xx} + w^{v}_{yy})) \\
+ G_{zz} (v^{v}_{xy} - v^{v}_{yy}) - \frac{194}{315} \frac{E_r}{h_t} \rho \omega \left( \frac{37h_t}{630} \rho r (\bar{w}^r + \bar{w}^p) \\
- \frac{17}{420} \rho (\bar{u}^r_{xx} + \bar{u}^r_{yy}) \right) + \frac{181}{6930} \rho \left( \frac{\partial^2}{\partial x^2} + \frac{\partial^2}{\partial y^2} \right) \left( w^w + \frac{199}{724} \bar{w}^v + \bar{w}^p \right) \\
- \frac{358}{105} E_r \left( 2w^w - w_x - w_y \right) - \frac{11}{15} G_{zz} (u^w_{xx} - u^w_{yy}) \\
+ \frac{11}{15} G_{zz} (v^w_{xy} - v^w_{yy}) = 0
\end{align}
\]

where,

\[
\alpha_4 = \frac{194}{315} h_t, \quad \alpha_0 = \frac{37}{630} \frac{h_t}{h_c} + \frac{383}{630} \left( \frac{h_t}{h_c} \right)^2.
\]

A similar set of equations for the motion of the bottom face sheet can be derived, as follows:

\[
\begin{align}
\partial_w \psi_0 &\quad N_{xx}^{v} + N_{yy}^{v} - \left( \rho h_t + \rho h_t \right) \frac{u^w_{xx} - \rho h_t}{6} v^w \\
- \rho \left( \frac{h_t}{420} \frac{23 \bar{w}^p + 17 \bar{w}^p}{2} - \bar{w}_x \right) - G_{zz} \left( \frac{1}{h_t} (u^w_{xx} - u^w_{yy}) \right) = 0,
\end{align}
\]

and

\[
\begin{align}
\partial_w \psi_0 &\quad N_{xy}^{v} + N_{yy}^{v} - \left( \rho h_t + \rho h_t \right) \frac{v^w_{xy} - \rho h_t}{6} v^w \\
- \rho \left( \frac{h_t}{420} \frac{23 \bar{w}^p + 17 \bar{w}^p}{2} - \bar{w}_x \right) - G_{zz} \left( \frac{1}{h_t} (v^w_{xy} - v^w_{yy}) \right) = 0.
\end{align}
\]

The corresponding boundary conditions at \( x = 0 \) are read as follows:

For the top face sheet:

\[
\begin{align}
u^w_0 &= \bar{u}^r \quad \text{or} \quad N^{v}_{xx} = \bar{N}^{v}_{xx}, \\
v^w_0 &= \bar{v}^r \quad \text{or} \quad N^{v}_{yy} = \bar{N}^{v}_{yy},
\end{align}
\]

where \( \bar{Q}^v_x \) is the resultant top face sheet shear, defined as the integral of \( \tau_{zz}/G_c \) over the top face sheet, and

\[
\begin{align}
\bar{w}^w_x &= \bar{w}^w_y \quad \text{or} \quad M^{v}_{xx} = \bar{M}^{v}_{xx}, \\
\bar{w}^w_y &= \bar{w}^w_x \quad \text{or} \quad M^{v}_{yy} = \bar{M}^{v}_{yy}.
\end{align}
\]

The superscript \( \sim \) denotes the known external boundary values.

Similar equations can be written for \( y = 0, b \). Assuming the sandwich plate is made of orthotropic materials and substituting Eq. (2b) into (6b) and (6c) and then into Eq. (10), one can rewrite the non-linear governing equations for the top face sheet as:

\[
\begin{align}
&\left( A^w_{11} u^w_{xx} + A^w_{00} u^w_{yy} + (A^w_{12} + A^w_{11}) v^w_{xy} - \frac{C^w_{zz}}{h_t} (u^w_0 - u^w_0) \right) \\
&\left( \rho \left( h_t + \frac{1}{3} \right) \bar{u}^r - \rho \frac{h_t}{6} \bar{u}^r + \rho h_t \bar{h}_t \frac{23 \bar{w}^p + 17 \bar{w}^p}{2} - \bar{w}_x \right) \\
&+ G_{zz} \left[ \frac{11}{15} (w^w_0 + x_0 (w^w_y + w^w_y)) \right] = \bar{F}^w_1,
\end{align}
\]

(18a)

\[
\begin{align}
&\left( A^w_{12} + A^w_{11} \right) u^w_{xx} + A^w_{00} u^w_{yy} + A^w_{12} v^w_{xy} - \frac{C^w_{zz}}{h_t} (v^w_0 - v^w_0) \\
&\left( \rho \left( h_t + \frac{1}{3} \right) \bar{v}^r - \rho \frac{h_t}{6} \bar{v}^r + \rho h_t \bar{h}_t \frac{23 \bar{w}^p + 17 \bar{w}^p}{2} - \bar{w}_x \right) \\
&+ G_{zz} \left[ \frac{11}{15} (w^w_0 + x_0 (w^w_y + w^w_y)) \right] = \bar{F}^w_2,
\end{align}
\]

(18b)

in which the last terms in the left-hand side of these equations reflect the effects of the higher order core theory, and the second from last terms in the left-hand side can be viewed as the excitation produced by the transverse motion for the in-plane motion; furthermore, the \( F^w_1, F^w_2 \) terms in the right-hand side represent the nonlinear terms, and these are:

\[
\begin{align}
- \alpha_1 h_t (G_{zz} w^{v}_{xx} + G_{zz} w^{v}_{yy}) - \alpha_0 (G_{zz} (u^{v}_{xx} - u^{v}_{yy})) \\
+ G_{zz} (v^{v}_{xy} - v^{v}_{yy}) - \frac{61}{21} \frac{E_r}{h_t} \left( w^{p}_{x} - \frac{358}{305} w^{p}_{y} + \frac{53}{305} w^{w} \right) = 0.
\end{align}
\]
\[ \ddot{F}_1 = -A_{y1}w_{x}w_{xx} - (A_{y2} + A_{y0})w_{y}w_{xy} - A_{y0}w_{y}w_{yy}, \]  
Equation (18c)

\[ \ddot{F}_2 = -(A_{x2} + A_{y0})w_{x}w_{xy} - A_{y0}w_{x}w_{y} - A_{x2}w_{y}w_{yy}. \]  
Equation (18d)

Furthermore, the third equation is:

\[ D_{11} w_{x}^{xxx} + 2(D_{11} + 2D_{60})w_{x}^{xyy} + D_{12} w_{y}^{yyy} \]
\[ + \frac{61 E}{21k_c} \left( \frac{w'}{2} \right) - \frac{358}{105} \frac{3}{0} \left( \frac{w''}{2} \right) + 29k_c \frac{99}{315} \left( \frac{w'}{2} \right)^3 \]
\[ + \rho h_c \frac{37h}{630} \left( \frac{w''}{2} \right) + \frac{99}{1155} \rho k_c \left( \frac{w'}{2} \right)^3 \]
\[ + \rho h_c \frac{37h}{27720} \left( \frac{w'}{2} \right)^3 \]
\[ + \rho h_c \frac{37h}{27720} \left( \frac{w'}{2} \right)^3 \]
\[ + \rho h_c \frac{37h}{27720} \left( \frac{w'}{2} \right)^3 \]

in which the right-hand sides are the nonlinear terms:

\[ \ddot{F}_3 = N_x^0 w_x^x + \left( N_x^0 w_x^x \right)_y + \left( N_x^0 w_x^x \right)_y + \left( N_x^0 w_x^x \right)_y. \]  
Equation (18f)

Similarly, one can also recast the equation for core as follows:

\[ x_2h_c(C_{y0}w_{x}w_{y}w_{xy}) + x_2h_c(C_{y0}w_{x}w_{x}w_{y}) + C_{y0}(w_{x}w_{y}) \]
\[ = \left( \rho h_c \frac{3}{0} \right) \frac{358}{105} \frac{3}{0} \left( \frac{w''}{2} \right) + \frac{99}{1155} \rho k_c \left( \frac{w'}{2} \right)^3 \]
\[ + \frac{37h}{630} \left( \frac{w''}{2} \right) + \frac{99}{1155} \rho k_c \left( \frac{w'}{2} \right)^3 \]
\[ + \frac{37h}{27720} \left( \frac{w'}{2} \right)^3 \]
\[ + \frac{37h}{27720} \left( \frac{w'}{2} \right)^3 \]
\[ + \frac{37h}{27720} \left( \frac{w'}{2} \right)^3 \]

Finally, for the bottom face sheet, the equations of motion become:

\[ A_{11}w_{x}^{xx} + A_{10}w_{x}^{xy} + (A_{12} + A_{10})w_{y}^{xy} + C_{x0}(u_{x} - u_{x}) \]
\[ - \left( \rho h_c \frac{3}{0} \right) \frac{358}{105} \frac{3}{0} \left( \frac{w''}{2} \right) + \frac{99}{1155} \rho k_c \left( \frac{w'}{2} \right)^3 \]
\[ + \frac{37h}{630} \left( \frac{w''}{2} \right) + \frac{99}{1155} \rho k_c \left( \frac{w'}{2} \right)^3 \]
\[ + \frac{37h}{27720} \left( \frac{w'}{2} \right)^3 \]
\[ + \frac{37h}{27720} \left( \frac{w'}{2} \right)^3 \]
\[ + \frac{37h}{27720} \left( \frac{w'}{2} \right)^3 \]

The displacements can be assumed as:

\[ u_{x} = \sum_{m,n} U_{x,m}(t) \cos \frac{m \pi x}{a} \sin \frac{n \pi y}{b}, \]  
Equation (21a)

\[ v_{x} = \sum_{m,n} V_{x,m}(t) \sin \frac{m \pi x}{a} \cos \frac{n \pi y}{b}, \]  
Equation (21b)

\[ w_{x} = \sum_{m,n} W_{x,m}(t) \sin \frac{m \pi x}{a} \sin \frac{n \pi y}{b}, \]  
Equation (21c)

\[ u_{y} = \sum_{m,n} U_{y,m}(t) \cos \frac{m \pi x}{a} \cos \frac{n \pi y}{b}, \]  
Equation (21d)

\[ v_{y} = \sum_{m,n} V_{y,m}(t) \sin \frac{m \pi x}{a} \sin \frac{n \pi y}{b}, \]  
Equation (21e)

\[ w_{y} = \sum_{m,n} W_{y,m}(t) \sin \frac{m \pi x}{a} \sin \frac{n \pi y}{b}, \]  
Equation (21f)

where \( U_{x,m}(t), V_{x,m}(t), U_{y,m}(t), V_{y,m}(t), W_{x,m}(t), \) and \( W_{y,m}(t) \) are unknown functions of time \( t \). These displacements satisfy the boundary conditions. Substituting the displacements (22) into the equations of motion (18)–(20), with \( \ddot{F}_1, \ddot{F}_2, \) and \( \ddot{F}_3 \) being expressed into the following form:

\[ \ddot{F}_1^{1h} = \sum_{nm} \ddot{F}_{1,m}(t) \cos \frac{m \pi x}{a} \sin \frac{n \pi y}{b}, \]  
Equation (23a)

\[ \ddot{F}_2^{1h} = \sum_{nm} \ddot{F}_{2,m}(t) \sin \frac{m \pi x}{a} \cos \frac{n \pi y}{b}, \]  
Equation (23b)
\( \vec{F}^b_{3} = \sum_{mn} \vec{F}^b_{3mn}(t) \sin \frac{m\pi x}{a} \sin \frac{n\pi y}{b}, \)
\( q(x,y,t) = \sum_{mn} \tilde{Q}_{mn}(t) \sin \frac{m\pi x}{a} \sin \frac{n\pi y}{b}. \)  

(23b)

we can obtain sets of second order ordinary differential equations with regard to the variable time in matrix form:

\[ [M_{mn}] U_{mn}(t) + [\kappa_{mn}] U_{mn}(t) \equiv F_{lmn}(t), \]

(24)

where \([M_{mn}]\) is the equivalent mass matrix, \([\kappa_{mn}]\) is the damping coefficient matrix and \([\kappa_{mn}]\) is the equivalent spring constant matrix. These are \(7 \times 7\) matrices for a given pair \((m,n)\).

The displacement vector \( U_{mn}\) is defined as \( U_{mn} = [U^b_{1mn}(t), V^b_{1mn}(t), W^c_{1mn}(t), W^b_{1mn}(t), V^b_{2mn}(t), W^b_{2mn}(t)]^T\) and the loading vector \( F_{mn} = [F_{1mn}(t) + \tilde{Q}_{mn}(t), F_{2mn}(t), F_{3mn}(t), 0, F_{1mn}(t), F_{2mn}(t), F_{3mn}(t)]^T. \)

\( \vec{U}(s) = (\tilde{U}(t)) = \int_{0}^{\infty} U(t)e^{-\delta t} dt \) 

to Eq. (24), one can further obtain:

\[ (s^2[M_{mn}] + s[\kappa_{mn}] + [\kappa_{mn}]) \tilde{U}_m(s) = \tilde{F}_{mn}(s). \]  

(27)

In the Laplace space, the solution in terms of the displacements to Eq. (27) can be obtained without much difficulty if the loading vector \( \vec{F}_{mn} = [F_{1mn} + \tilde{Q}_{mn}(s), F_{2mn} + \tilde{Q}_{mn}(s), 0, F_{1mn} + \tilde{Q}_{mn}(s), F_{2mn} + \tilde{Q}_{mn}(s), 0] \) is constant, then (27) is a set of linear algebraic equations, which can be solved directly for \( \tilde{U}_{mn} = [U^b_{1mn}, U^b_{2mn}, W^c_{1mn}, W^b_{1mn}, V^b_{2mn}, W^b_{2mn}] \) and then the displacements in time domain \( U_{mn} = [U^b_{1mn}(t), U^b_{2mn}(t), W^c_{1mn}(t), W^b_{1mn}(t), V^b_{2mn}(t), W^b_{2mn}(t)]^T \) can be recovered using the inverse Laplace Transform without much difficulty. Subsequently, the solution for the displacements can be found by using Eqs. (22). But the loading coefficients \( \vec{F}^b_{lmn} \) and \( \tilde{F}_{lmn} \) were derived from the expressions \((18c),(18d),(18f)\) and \((20d)-(20f)\), which are non-linear functions of the displacements. Therefore, the right-hand side of Eq. (27), \( \vec{F}_{mn} \) are non-linear functions of \( \tilde{U}_{mn}. \) Therefore, an iterative procedure is developed as follows: (1) First, \( \tilde{Q}_{mn} \) is a known function once the applied load is given. If the right-hand side of Eq. (27) is approximated by \( \vec{F}_{mn} = \tilde{Q}_{mn}(s), 0, 0, 0, 0, 0 \), then a first approximation to the solution is easily obtained as \( \{U_{mn}(s) = s^2[M_{mn}] + s[\kappa_{mn}] + [\kappa_{mn}])^{-1} \tilde{F}_{mn}(s). \) (2) Application of the Inverse Laplace Transform to \( \tilde{U}_{mn}(s) \) lead to the corresponding solution \( U_{mn}(t). \) Then, making use of Eqs. \((18c),(18d),(18f),(20d)-(20f)\) and (22), one can determine the functions \( F_{1}, F_{2}, F_{3} \) and \( F_{1}, F_{2}, F_{3} \) and then the corresponding to these Laplace Transforms \( F_{1}, F_{2}, F_{3} \) and \( F_{1}, F_{2}, F_{3}. \) (3) The next approximation for the displacements is found by solving Eq. (27) with the updated vector \( \vec{F}_{mn} = [F_{1mn} + \tilde{Q}_{mn}(s), F_{2mn} + \tilde{Q}_{mn}(s), 0, F_{1mn} + \tilde{Q}_{mn}(s), F_{2mn} + \tilde{Q}_{mn}(s), 0] \) and \( \epsilon \leq 10^{-5} \) between two consecutive steps.

It should be mentioned that, in general, an iterative procedure combined with the Laplace transform in time to solve a set of non-linear equations, may converge or diverge depending on the coefficient matrices and the applied loading amplitudes. For practical structural configurations, the displacement solution is expected to converge until the dynamic buckling phenomenon occurs. In this study, in which we produce results for a realistic sandwich structure, we found that the solution is convergent after only six iterations.

5. Numerical results and discussions

In this section, we shall present the numerical results for typical sandwich plates with orthotropic phases. Since the sandwich structure consist of orthotropic phases, the relationship for the Poisson’s ratios as: \( \nu_{ij} = \nu_{ji}, \) will be applied without explicit explanation. Let us first consider faces with elastic constants (in GPA): \( E_1 = 40.0, E_2 = 10.0, E_3 = 10.0, G_{12} = 4.5, G_{23} = 3.5, G_{31} = 4.5; \) Poisson’s ratios: \( \nu_{12} = 0.065, \nu_{13} = 0.260, \nu_{23} = 0.400 \) (these are typical of glass/epoxy composite). The orthotropic core has elastic constants reading as (in GPA): \( E_1 = E_2 = 0.032, E_3 = 0.30, G_{12} = 0.013, G_{23} = 0.048, G_{31} = 0.048; \) Poisson’s ratios: \( \nu_{12} = \nu_{13} = \nu_{23} = 0.25 \) (these are typical of honeycomb material).

In the following we denote by \( h_{tot}\) the total thickness of the plate, defined as \( h_{tot} = 2h_1 + h_c. \) In the example presented the face sheet thickness is \( h_1 = 2\text{ mm} \) and the core thickness \( h_c = 18\text{ mm}. \) The plate dimensions are: \( a = 25h_{tot} \) and \( b = 50h_{tot}. \) The top face of the sandwich plate is assumed to be blasted by an impulsive load at the point \((x_0,y_0),\) which is of the following form:

\[ p(x,y) = p_0(t) \delta(x-x_0) \delta(y-y_0). \quad 0 < x-x_0 < a, \quad 0 < y-y_0 < b. \]

(28a)

where \( \delta \) is the delta function. The intensity of the loading varies with time exponentially as \( p_0(t) = Q_0e^{-t/\tau} \) MPa, \( \tau > 0. \)

(28b)

in which we use the values from Librescu et al. (2004): \( Q_0 = 60.86 \) and \( x = 3.33435. \) From Eq. (25a) one can obtain the following loading in the transformed space:

\[ Q_{mn} = \frac{4}{ab} p_0(t) \sin \frac{m\pi x_0}{a} \sin \frac{n\pi y_0}{b}. \quad 0 < x_0 < a, \quad 0 < y_0 < b. \]

(28c)

This loading case is the idealization of an explosive impact near a big sandwich plate. We shall study the case of \( x_0 = 0.5a, y_0 = 0.5b. \) However, all the methods and procedures can be extended for an arbitrary loading location of \((x_0,y_0).\)

Regarding the number of terms used in producing the results, we have used \( m = 10 \) and \( n = 10. \) No noteworthy difference in the results was found with a larger number of terms. In fact, we found that the calculations converge when \( m \geq 8 \) and \( n \geq 8, \) with no real difference beyond these levels of \( m \) and \( n. \)

Plotted in Fig. 2 is the distribution of the normalized transverse displacement as functions of \( y \) at \( x = 0.5a \) in the face sheets and middle plane of the core at time instants \( t = 25\mu s \) and \( 250\mu s, \) respectively. The results demonstrate the evolution of motion propagating outward from the center points when the point-wise blast loading impacts on the top face at its center. Since the energy is still added into the sandwich material system from \( t = 25\mu s \) and \( t = 250\mu s, \) the maximum amplitudes of these displacements increase during the evolution. One can also apparently see that the transverse displacements in the top face, middle plane of the core and bottom face are not identical. This observation is in good agreement with those in Li et al. (2008). A three dimensional displacement distribution profile for the top face sheet is presented.
in Fig. 3 for time instant $t = 0.45$ ms, or 450 $\mu$s. As expected, the maximum displacement occurs at the center point where the loading is applied.

Fig. 4 depicts the transient displacements for 4 different points $(0.5a, 0.5b), (0.25a, 0.5b), (0.5a, 0.25b)$ and $(0.25a, 0.25b)$ in the top face. Since $a=b$, the curves for $(0.25a, 0.5b)$ and $(0.5a, 0.25b)$ are not identical. One can further see that the displacements for the points $(0.25a, 0.5b), (0.5a, 0.25b)$ and $(0.25a, 0.25b)$ could be tangled when $t \leq 10$ ms (in the sense that the displacement of a further away point can even exceed that of a closer point), but these displacements decrease as the distance from the center point increases for each time instant of $t > 10$ ms. However, the maximum displacement occurs at the center when $t = 2.25$ ms.

Presented in Figs. 5–7 are the stress profiles in the core at three locations: $(x = 0.5a, y = 0.5b), (x = 0.375a, y = 0.375b)$ and $(x = 0.25a, y = 0.25b)$, respectively. These profiles are used to demonstrate the stress distribution in the core through the thickness direction and as a function of time. In these figures $z/h_c = 0.5$ is along the interface between the bottom face and the core, and $z/h_c = -0.5$ is the interface between the core and the top face sheet, on which the impulsive impact is exerted. Fig. 5 shows that the stress is always negative (compressive) at the top face/core.
interface for \( (x = 0.5a, y = 0.5b) \), but could be positive at the bottom face/core interface. One may also see that the maximum amplitude of the stress happens at the top face/core interface, as expected. These observations agree with the ones in Li et al. (2008), where the loading is uniformly distributed over the top face. However, at any position other than \( (x = 0.5a, y = 0.5b) \), the stress could be either positive or negative at either of these interfaces, as shown in Figs. 6 and 7. One may also easily see that the maximum values in the transverse stress in the core of the sandwich plate decrease dramatically when \( x \) and \( y \) are away from \( (0.5a, 0.5b) \).

The effects from the ratio of face sheet thickness over core thickness, \( h_f/h_c \), on the transient response are illustrated in Fig. 8. In these results, the thickness of the plate is kept constant at 20mm. It can be seen that when \( h_f/h_c \) increases, the maximum value of the displacement at the center of the top face decreases. This observation has significance in the design of a sandwich structure. For example, if the maximum transverse deformation is the design criterion for certain sandwich structures, one must ensure the ratio \( h_f/h_c \) shall not be less than a critical value. In the meanwhile, this \( h_f/h_c \) may be as small as possible in order to ensure adequate weight savings. Therefore, the results can provide a guideline for optimal design.

6. Conclusions

The transient response of an orthotropic composite sandwich plate subject to point-wise impulse (blast) loading is studied using a nonlinear high order core theory. It is found that the top face, the core and the bottom face behave differently in the transient response. The transverse stress profiles in the core show high nonlinearity with maximum amplitudes at the interface between the core and top face sheet on which the blast loading impacts. Therefore, debonding could initiate at this interface as has been observed in preliminary experiments. The stress and displacement amplitudes decrease very rapidly away from the point loading. The effect of the ratio of face thickness over core thickness is analyzed and these observations could suggest some guidelines for sandwich plate optimal design.

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